

RAMEY KEMP & ASSOCIATES, INC.

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November 16, 2017

Mr. Michael Zuraf, AICP Stafford County Planning and Zoning 1300 Courthouse Road Stafford, Virginia 22554 Phone: (540) 658-8668

Reference: Holly Corner Manor – Traffic Impact Analysis

Stafford County, Virginia

Dear Mr. Zuraf,

Ramey Kemp & Associates, Inc. (RKA) has performed a Traffic Impact Analysis (TIA) for this proposed neighborhood, which consists of up to 130 single-family lots on the south side of Holly Corner Road approximately one mile west of U.S. 17 (Warrenton Road). The proposed access plan includes one new full-movement driveway on Holly Corner Road, and cross access to the intersection of Country Manor Drive at Wild Rose Drive. If approved, the neighborhood is expected to be built-out by 2022. Figure 1 shows the site location and study intersections.

Based on the February 8 scoping meeting with the County and the Virginia Department of Transportation (VDOT), the purpose of this letter report is to provide the following:

- Trip generation calculations
- Evaluation of turn lane warrants for the proposed driveway on Holly Corner Road
- Sight distance evaluation at proposed driveway location
- Capacity analysis of the study intersections

Existing Roadway Conditions

U.S. 17 (Warrenton Road) is a four-lane divided Principal Arterial with an average daily traffic (ADT) volume of approximately 21,500 vehicles per day south of Holly Corner Road, and an ADT volume of approximately 20,500 vehicles per day north of Holly Corner Road. Warrenton Road has a posted speed limit of 45 mph.

Route 655 (Holly Corner Road) is a two-lane Minor Collector with an ADT volume of approximately 2,000 vehicles per day, and a posted speed limit of 35 mph.

Existing Traffic Volumes

The AM peak hour (7:00 to 9:00 AM) and PM peak hour (4:00 to 6:00 PM) turning movement counts were conducted by Technical Traffic Services, LLC at the following intersection during the week of February 13:

U.S. 17 at Holly Corner Road

Figure 2 shows the existing 2017 traffic volumes and the count data is enclosed. For the analysis, the peak hour factor (PHF) was calculated by approach.

Background Traffic Growth

Based on discussion at the scoping meeting, the existing volumes were grown by an annual rate of 1.5% for five years. Figure 2 shows the projected 2022 peak hour volumes with only growth applied.

Approved Development Traffic

We understand there are three approved developments near the site that are included in this TIA:

- The Westlake neighborhood consists of 700 single family lots and is located west of U.S. 17 between Richards Ferry Road and Cedar Grove Road. Based on discussion at the scoping meeting, it was assumed that 200 of the homes will be occupied by 2022.
- Summerset Ridge South consists of 23 single family homes located on Holly Corner Road west of the site.
- Holly Refuge consists of 7 single family homes located on Holly Corner Road west of the site.

The trip generation potential of the approved developments during a typical weekday, AM peak hour and PM peak hour was estimated using the methodologies published by the Institute of Transportation Engineers (ITE) *Trip Generation Manual* -9^{th} *Edition*, and are shown in Table 1.

Table 1
ITE Trip Generation – 9th Edition – Weekday – Approved Developments

Land Use (ITE Land Use Code)	Size	Daily '	kday Fraffic od)	AM Pea (vp	k Hour	PM Peak Hour (vph)		
		Enter	Exit	Enter	Enter Exit		Exit	
		Westla	ıke					
Single Family Detached (210)	200 homes	952	952	38	112	126	74	
	Sumr	nerset Ri	dge Sout	h				
Single Family Detached (210)	23 homes	110	110	4	13	14	9	
		Holly Re	fuge					
Single Family Detached (210)	7 homes	34	34	1	4	4	3	



Based on discussion with the County and VDOT, all of the approved developments are assumed to have the same traffic distribution. Figure 3 shows the trip assignment for Westlake, Summerset Ridge South, and Holly Refuge. The total approved development trips are shown in Figure 4. The total approved development trips were combined with the background growth to estimate the 2022 no-build traffic volumes, which are shown in Figure 4.

Trip Generation

Table 2 shows the ITE trip generation potential of the proposed Holly Corner Manor neighborhood.

Table 2
ITE Trip Generation – 9th Edition – Weekday – Holly Corner Manor

Land Use (ITE Land Use Code)	Size	Daily Traine				k Hour h)	
		Enter	Exit	Enter	Exit	Enter	Exit
Single Family Detached (210)	130 homes	668	668	25	76	84	49

Site Traffic Distribution

The following site traffic distribution was applied based on a review of the existing traffic volumes, the adjacent roadway network, and engineering judgment:

- 85% to / from the south on U.S. 17
- 13% to / from the north on U.S. 17
- 2% to / from the west on Holly Corner Road

Figures 5 and 6 show the site trip distribution and site trip assignment. Figure 7 shows the projected build 2022 AM and PM peak hour traffic volumes.

Intersection Spacing Standards

VDOT requires at least 335 feet of separation between full-movement intersections on two-lane Minor Collector roadways posted 35 mph. The proposed driveway on Holly Corner Road is approximately 850 feet east of Cricket Lane and approximately 1,175 feet west of Country Manor Drive, which exceeds VDOT's minimum intersection spacing standards in both directions.



Traffic Capacity Analysis

Traffic capacity analysis was performed using Synchro 9.1, which is a comprehensive software package that allows the user to model signalized and unsignalized intersections to determine levels-of-service based on the thresholds specified in the 2010 Highway Capacity Manual (HCM).

Table 3 summarizes the capacity analysis results for the unsignalized intersection of U.S. 17 at Holly Corner Road, and all of the Synchro outputs are enclosed for reference.

Table 3
Level-of-Service Summary
U.S. 17 at Holly Corner Road (Unsignalized)

	LANE	Al	M PEAK H	IOUR	PN	1 РЕАК Н	OUR
CONDITION	GROUP	Lane LOS	Queue (ft)	Overall LOS (Delay)	Lane LOS	Queue (ft)	Overall LOS (Delay)
Existing 2017 Traffic Conditions	EBL/R ¹ NBL ² NBT SBT SBR	F B - -	138 10 - -	N/A ³	E B - -	48 23 - -	N/A ³
No-Build 2022 Traffic Conditions	EBL/R ¹ NBL ² NBT SBT SBR	F B - -	298 8 - -	N/A ³	F C - -	175 35 - -	N/A ³
Build 2022 Traffic Conditions	EBL/R ¹ NBL ² NBT SBT SBR	F B - -	505 10 - - -	N/A ³	F C - -	413 60 - -	N/A ³
Build 2022 Traffic Conditions (With Recommended Improvement)	EBL ¹ EBR ¹ NBL ² NBT SBT SBR	F C B -	160 65 10 - -	N/A ³	F C C -	98 45 60 - -	N/A ³

- 1. Level of service for minor approach
- 2. Level of service for major street left-turn movement
- 3. HCM methodology does not provide lane group or overall LOS, delay, and queue lengths for major street through movements or right turns at unsignalized intersections.

Capacity analysis indicates that the minor street left-turn movement currently operates with long delays (greater than 50 seconds) during the AM peak hour and moderate delays (between 25 and 50 seconds) during the PM peak hour. Under no-build conditions, the minor street left-turn movement is expected to operate with long delays (greater than 50 seconds) during both peak hours. Under build conditions, the minor street left-turn movement is expected to continue to operate with long delays (greater than 50 seconds) during both the AM and PM peak hours. To mitigate queuing on the eastbound Holly Corner Road approach, the following improvement is recommended:

Construct an eastbound right-turn lane on Holly Corner Road with 150 feet of storage



Long delays are common for minor street left-turn movements at intersections with major thoroughfares, and this intersection is not expected to meet signal warrants.

Note that this analysis is conservative, because the Synchro model does not include the traffic signals on U.S. 17, which create gaps for drivers turning off Holly Corner Road.

Table 4 summarizes the capacity analysis results for the unsignalized intersection of Holly Corner Road at Proposed Site Driveway, and all of the Synchro outputs are enclosed for reference.

Table 4
Level-of-Service Summary
Holly Corner Road at Proposed Site Driveway (Unsignalized)

	LANIE	Al	M PEAK H	IOUR	PM	I PEAK H	OUR
CONDITION	LANE GROUP	Lane LOS	Queue (ft)	Overall LOS (Delay)	Lane LOS	Queue (ft)	Overall LOS (Delay)
Build 2022 Traffic Conditions	EBT/R WBL/T ² NBL/R ¹	- A A	- 0 8	N/A ³	- A A	- 5 3	N/A ³

- 1. Level of service for minor approach
- 2. Level of service for major street left-turn movement
- HCM methodology does not provide lane group or overall LOS, delay, and queue lengths for major street through
 movements or right turns at unsignalized intersections.

The capacity analysis indicates that all movements are projected to operate with short delays (less than 25 seconds) during the AM and PM peak hours at build-out of the neighborhood, and the queue lengths are expected to be one vehicle or less.

VDOT Turn Lane Warrant Analysis

The projected build-out AM and PM peak hour traffic volumes at the proposed driveways were compared to the turn lane warrants in the Virginia Department of Transportation (VDOT) Access Management Design Standards for Entrances and Intersections:

- A westbound left-turn lane on Holly Corner Road is <u>not</u> warranted
- An eastbound right-turn lane or taper on Holly Corner Road is not warranted

The turn lane warrant diagrams are enclosed for reference.



Sight Distance Evaluation

The sight distance for drivers exiting the proposed neighborhood driveway were measured in the field. Table 5 summarizes the sight distance requirements for driveways on two-lane roadways according to the VDOT Road Design Manual and according to *A Policy on Geometric Design of Highways and Streets*, which is published by the American Association of State Highway and Transportation Officials (AASHTO).

Table 5
Sight Distance Requirements
(From Exhibits 9-55 and 9-58 in the 2004 AASHTO Green Book)

Design Speed	Turn	VDOT Minimum Sight Distance	AASHTO Minimum Sight Distance	Actual Sight Distance
Holly Corner Road:	Left	390 feet	445 feet	625 feet
40 mph	Right	390 feet	385 feet	475 feet

At the proposed driveway location on Holly Corner Road, drivers exiting the neighborhood will be able to see approximately 625 feet to the east, and 475 feet to the west, which exceeds sight distance minimums in both directions. Enclosed for reference are photos taken approximately 275 feet east of the proposed driveway location looking in both directions.

Recommendations

Based on the trip generation potential of the proposed neighborhood, the following off-site roadway improvements are recommended:

U.S. 17 at Holly Corner Road:

Construct one eastbound right turn-lane on Holly Corner Road with 150 feet of storage

Holly Corner Road at Proposed Site Driveway:

Provide one ingress lane and at least one egress lane on the proposed site driveway

Figure 7 shows the recommended roadway improvement.



Mr. Michael Zuraf, AICP Page 7 of 7

We appreciate your attention to this matter. Please contact me at (804) 217-8560 if you have any questions

about this report.

Sincerely yours,

Ramey Kemp & Associates, Inc.

Carl Hultgren, P.E., PTOE Regional Manager

Enclosures: Figures, Traffic count data, Synchro output, VDOT turn lane warrant diagrams, Sight distance

photos

Copy to: Ms. Margaret Niemann, VDOT

Mr. Samer Shalaby, P.E., Development Consulting Services, PLC

Mr. Justin Franklin, P.E., Fairbanks & Franklin







Inset

LEGEND



Study Intersection



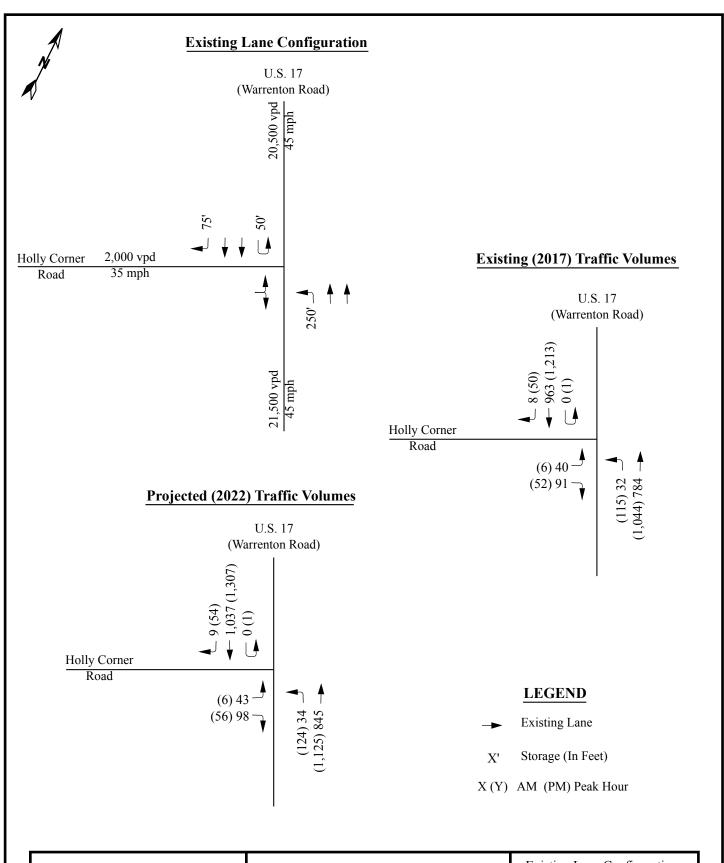
Site Boundary





Holly Corner Manor Stafford County, Virginia Site Location and Study Intersections

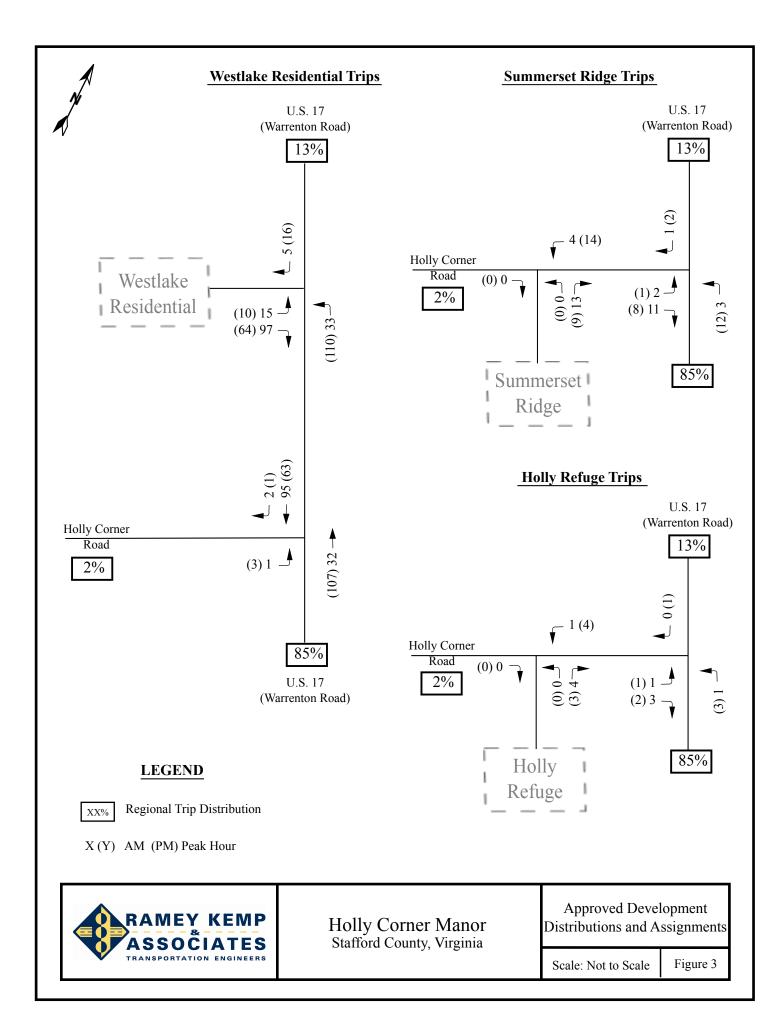
Scale: Not to Scale





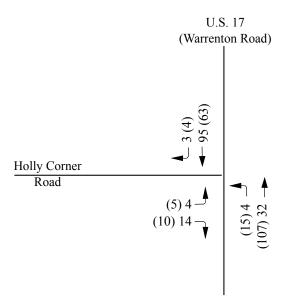
Holly Corner Manor Stafford County, Virginia Existing Lane Configuration, Existing (2017) Traffic Volumes, Projected (2022) Traffic Volumes

Scale: Not to Scale

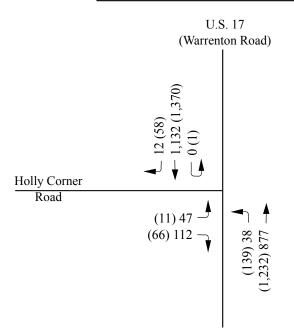




Total Approved Development Trips



No-Build (2022) Traffic Volumes



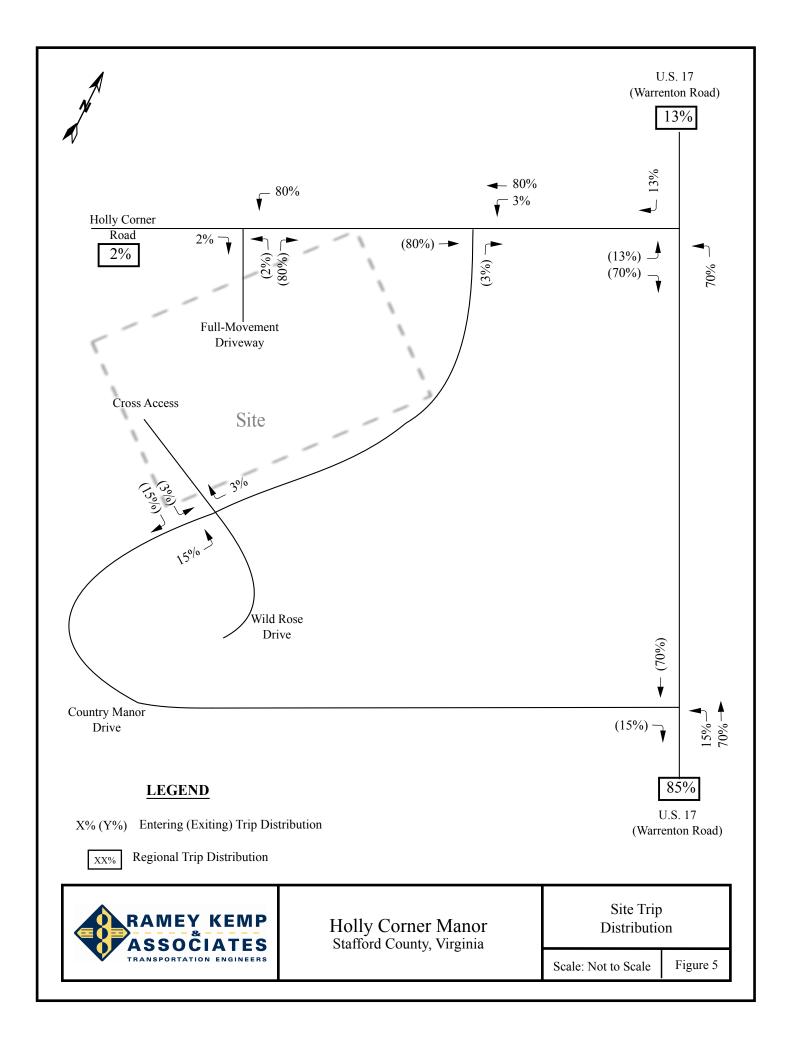
X (Y) AM (PM) Peak Hour

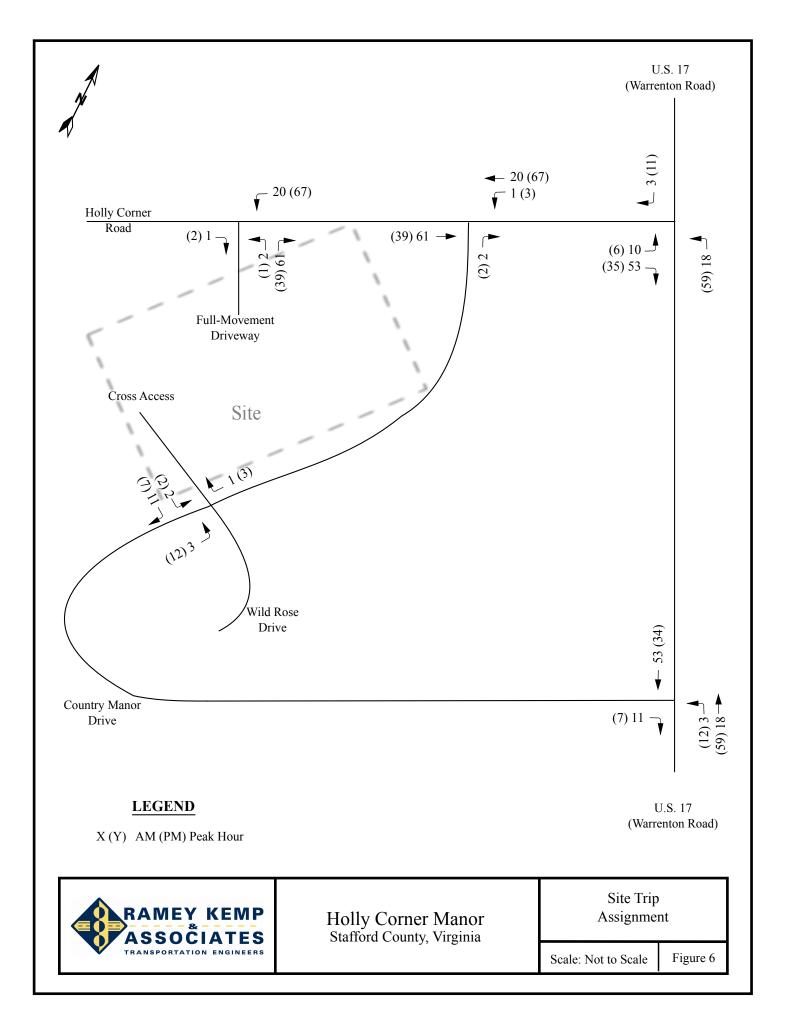
LEGEND

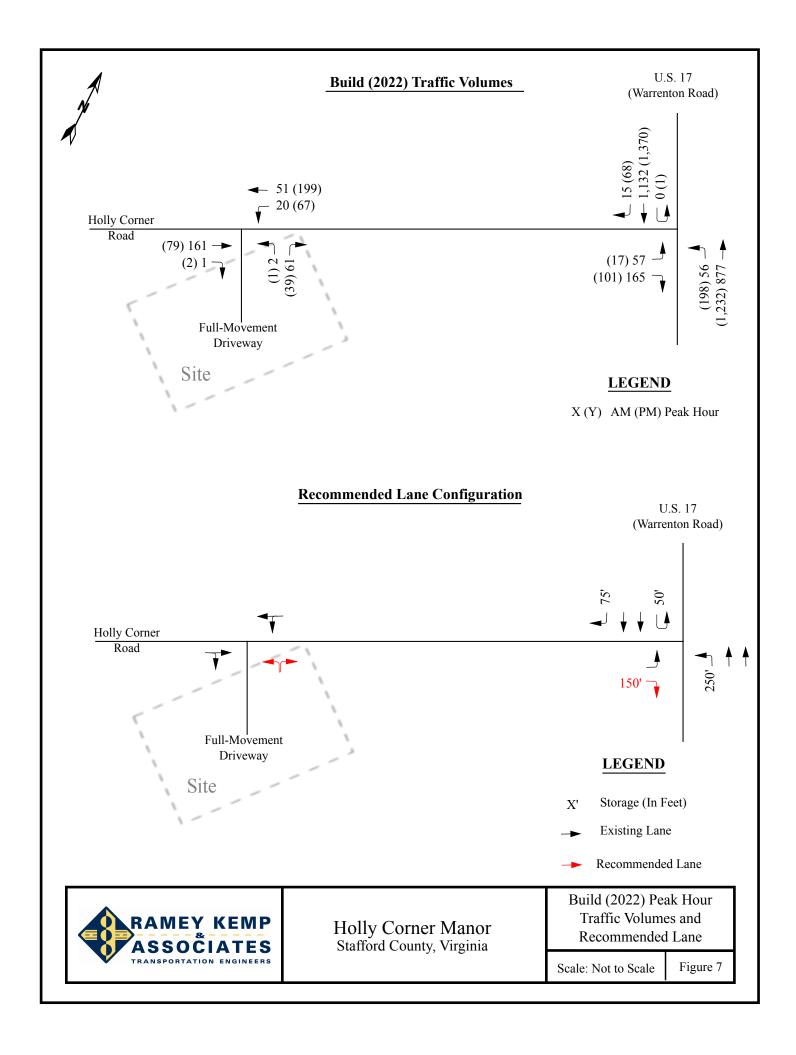


Holly Corner Manor Stafford County, Virginia Total Approved Development Traffic and No-Build (2022) Peak Hour Traffic Volumes

Scale: Not to Scale







Directional Turning Movement Study (7:00 AM-9:00 AM)

Locatio	n: US 1	7 (Warr	enton R	d.) and	d SR 65	5 (Holly	/ Corne	er Rd.)		Coun	ty/Ar	ea: St	afford	Count	у		
Date Su	irveyed	l: Februa	ary 14, 2	2017						Weat	ther: (Clear/	Sunny				
		US				US :				N	Α		Holly		r Rd. (SR	655)	
		From I	North			From S	outh			From East				From	West		
End Time	U-Turn	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Int. Total
7:15	0	187	3	0	8	200		0					23		21	0	442
7:30	0	224	0	0	5	171		0					17		31	0	448
7:45	0	280	4	0	8	179		0					16		23	0	510
8:00	0	245	3	0	5	170		0					5		27	0	455
8:15	0	208	0	0	7	227		0					10		24	0	476
8:30	0	230	1	0	12	208		0					9		17	0	477
8:45	0	244	8	0	12	154		0					10		22	0	450
9:00	0	262	1	0	12	187		0					11		21	0	494
Total		1880	20	0	69	1496		0					101		186	0	3752
% Appr Total		98.9%	1.1%		4.4%	95.6%							35.2%		64.8%		

Directional Turning Movement Study (4:00 PM-6:00 PM)

Locatio	n: US 1	7 (War	rentor	Rd.)	and SR	655 (Ho	lly Cor	ner Rd.)	County	/Area: Si	tafford (County				
Date Su	rveyed	l: Febru	ary 14	, 201	7					Weathe	er: Clear,	Sunny					
		US 1				US 1 From S					IA n East		Holly	Corner From	Rd. (SR 6	555)	~
End Time	U-Turn	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Left	Thru	Right	Ped	Int. Total
16:15	0	285	15	0	19	255		0					11		24	0	609
16:30	0	280	13	0	29	270		0					2		13	0	607
16:45	0	272	11	0	15	287		0					2		9	0	596
17:00	0	287	10	0	27	268		0					2		11	0	605
17:15	1	293	13	0	24	245		0					0		21	0	597
17:30	0	309	10	0	36	288		0					3		12	0	658
17:45	0	298	17	0	26	255		0					1		8	0	605
18:00	0	313	10	0	29	256		0					2		11	0	621
Total	1	2337	99	0	205	2124		0					23		109	0	4898
% Appr Total	0.04%	95.9%	4.1%		8.80%	91.20%							17.4%		82.6%		

Intersection							
	5.1						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	A		*	^	^	7	
Traffic Vol, veh/h	40	91	32	784	963	8	
Future Vol, veh/h	40	91	32	784	963	8	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	250	-	-	75	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	84	84	87	87	86	86	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	48	108	37	901	1120	9	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	1644	560	1120	0	-	0	
Stage 1	1120	-	-	-	-	-	
Stage 2	524	-	-	-	-	-	
Critical Hdwy	6.84	6.94	4.14	-	-	-	
Critical Hdwy Stg 1	5.84	-	-	-	-	-	
Critical Hdwy Stg 2	5.84	_	-	_	-	_	
Follow-up Hdwy	3.52	3.32	2.22	_		_	
Pot Cap-1 Maneuver	90	472	619	_		_	
Stage 1	274	-	-	_	_	_	
Stage 2	559	_	_	_	_	_	
Platoon blocked, %	007			_	_	_	
Mov Cap-1 Maneuver	85	472	619	_	_	_	
Mov Cap-2 Maneuver	85	-	-	_	_	_	
Stage 1	274	_	_	_	_	_	
Stage 2	526	_		_	_		
Stage 2	320						
Approach	EB		NB		SB		
HCM Control Delay, s	69.7		0.4		0		
HCM LOS	67.7 F		0.4		U		
TIOW LOO	'						
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR				
Capacity (veh/h)	619	- 197					
HCM Lane V/C Ratio	0.059	- 0.792					
HCM Control Delay (s)	11.2	- 69.7					
HCM Lane LOS	В	- F					
HCM 95th %tile Q(veh)	0.2	- 5.5					
110m 70m 70mc Q(VCH)	0.2	3.3					

Intersection							
nt Delay, s/veh	1.8						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥		ሻ	^	^	7	
Traffic Vol, veh/h	6	52	115	1044	1213	50	
Future Vol, veh/h	6	52	115	1044	1213	50	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	250	-	-	75	
/eh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	69	69	89	89	98	98	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	9	75	129	1173	1238	51	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	2083	619	1238	0	-	0	
Stage 1	1238	_	-	-	-	_	
Stage 2	845	_	-	-	-	_	
Critical Hdwy	6.84	6.94	4.14	-	-	_	
Critical Hdwy Stg 1	5.84	_	-	-	-	_	
Critical Hdwy Stg 2	5.84	-	-	-	-	-	
Follow-up Hdwy	3.52	3.32	2.22	-	-	-	
Pot Cap-1 Maneuver	46	432	558	-	-	-	
Stage 1	237	-	-	-	-	-	
Stage 2	382	-	-	-	-	-	
Platoon blocked, %				-	-	_	
Nov Cap-1 Maneuver	35	432	558	-	-	_	
Mov Cap-2 Maneuver	35	-	-	-	-	-	
Stage 1	237	-	-	-	-	-	
Stage 2	294	_	-	-	-	-	
J							
Approach	EB		NB		SB		
HCM Control Delay, s	35.7		1.3		0		
HCM LOS	Ε						
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR				
Capacity (veh/h)	558	- 199					
HCM Lane V/C Ratio	0.232	- 0.422					
HCM Control Delay (s)	13.4	- 35.7					
HCM Lane LOS	В	- E					
HCM 95th %tile Q(veh)	0.9	- 1.9					
2 (3.1)	3.7	,					

Intersection								
nt Delay, s/veh 18	8.9							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	W		*	^	^	7		
Traffic Vol, veh/h	47	112	38	877	1132	12		
Future Vol, veh/h	47	112	38	877	1132	12		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	250	-	-	75		
Veh in Median Storage, #	0	-	-	0	0	-		
Grade, %	0	-	_	0	0	-		
Peak Hour Factor	84	84	87	87	86	86		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	56	133	44	1008	1316	14		
Major/Minor	Minor		Major1		Malara			
Major/Minor	Minor2	450	Major1	0	Major2	0		—
Conflicting Flow All	1907	658	1316	0	-	0		
Stage 1	1316	-	-	-	-	-		
Stage 2	591	- (0.4	-	-	-	-		
Critical Hdwy	6.84	6.94	4.14	-	-	-		
Critical Hdwy Stg 1	5.84	-	-	-	-	-		
Critical Hdwy Stg 2	5.84	-	-	-	-	-		
Follow-up Hdwy	3.52	3.32	2.22	-	-	-		
Pot Cap-1 Maneuver	60	407	521	-	-	-		
Stage 1	215	-	-	-	-	-		
Stage 2	516	-	-	-	-	-		
Platoon blocked, %				-	-	-		
Mov Cap-1 Maneuver	~ 55	407	521	-	-	-		
Mov Cap-2 Maneuver	~ 55	-	-	-	-	-		
Stage 1	215	-	-	-	-	-		
Stage 2	472	-	-	-	-	-		
			=					
Approach	EB		NB		SB			
HCM Control Delay, s	253.7		0.5		0			
HCM LOS	F							
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR					
			אטכ וטכ					
Capacity (veh/h)	521	- 141						
HCM Cantral Dalay (a)	0.084	- 1.342						
HCM Control Delay (s)	12.5	- 253.7						
HCM Lane LOS	В	- F						
HCM 95th %tile Q(veh)	0.3	- 12						
Notes								
-: Volume exceeds capaci	ty \$: De	lay exceeds 30	00s +: Com	putation No	ot Defined *: All major v	volume in p	latoon	

Intersection							
nt Delay, s/veh	7.8						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
_ane Configurations	¥		ሻ	^	^	7	
Fraffic Vol, veh/h	11	66	139	1232	1370	58	
uture Vol, veh/h	11	66	139	1232	1370	58	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
T Channelized	'-	None	-	None	-	None	
Storage Length	0	-	250	-	-	75	
/eh in Median Storage, #	0	_	-	0	0	-	
Grade, %	0	-	-	0	0	_	
Peak Hour Factor	69	69	89	89	98	98	
Heavy Vehicles, %	2	2	2	2	2	2	
Nymt Flow	16	96	156	1384	1398	59	
WWW. From	10	70	100	1001	1070	07	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	2402	699	1398	0		0	
Stage 1	1398	-	-	-	-	-	
Stage 2	1004	_	_	_	_	_	
Critical Hdwy	6.84	6.94	4.14	_	_	_	
Critical Hdwy Stg 1	5.84	-	-	_	_	_	
Critical Hdwy Stg 2	5.84	_	_	_	_	_	
Follow-up Hdwy	3.52	3.32	2.22	_	_	_	
Pot Cap-1 Maneuver	28	382	485	_	_	_	
Stage 1	194	-	-	_	_	_	
Stage 2	315	_	_	_	_	_	
Platoon blocked, %	313			_	_	_	
Nov Cap-1 Maneuver	19	382	485	_	_	_	
Mov Cap-2 Maneuver	19	302	-	_	_	_	
Stage 1	194						
Stage 2	214	-	-	-	-	-	
Stage 2	214	-	-	-	-	-	
approach	EB		NB		SB		
HCM Control Delay, s	195		1.6		0		
HCM LOS	F		1.0		U		
IOWI LOO	1						
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR				
Capacity (veh/h)	485	- 102					
HCM Lane V/C Ratio	0.322	- 1.094					
HCM Control Delay (s)	15.9	- 195					
HCM Lane LOS	C	- F					
HCM 95th %tile Q(veh)	1.4	- 7.1					
TOWN FORTH FORTH CONTROL	1.7	7.1					

Intersection							
nt Delay, s/veh	16.5						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	¥		ሻ	^	^	7	
Traffic Vol, veh/h	57	165	56	877	1132	15	
Future Vol, veh/h	57	165	56	877	1132	15	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	-	250	-	-	75	
/eh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	84	84	87	87	86	86	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	68	196	64	1008	1316	17	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	1949	658	1316	0		0	
Stage 1	1316	-	-	-	-	-	
Stage 2	633	_	_	-	-	_	
Critical Hdwy	6.84	6.94	4.14	_	_	_	
Critical Hdwy Stg 1	5.84	-	-	_	_	_	
Critical Hdwy Stg 2	5.84	_	_	_	_	_	
Follow-up Hdwy	3.52	3.32	2.22	_	_	_	
Pot Cap-1 Maneuver	~ 56	407	521	_	_	_	
Stage 1	215	-	521	_	_	_	
Stage 2	491	_		_	_	_	
Platoon blocked, %	7/1						
Mov Cap-1 Maneuver	~ 49	407	521				
Mov Cap-1 Maneuver	~ 49	407	J2 I	_			
Stage 1	215	-	-	-	-	-	
Stage 2	431	-	-	-	-	-	
Stage 2	431	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	\$ 466.6		0.8		0		
HCM LOS	F						
Ainor Lano/Major Mymt	NIDI	NBT EBLn1	SBT SBR				
Minor Lane/Major Mvmt	NBL 521						
Capacity (veh/h)	521	- 142					
HCM Lane V/C Ratio	0.124	- 1.861					
HCM Control Delay (s)	12.9	-\$ 466.6					
HCM Lane LOS	В	- F					
HCM 95th %tile Q(veh)	0.4	- 20.2					
Votes							

Intersection								
	12							
Movement	EBL	EBR	NBL	NBT		SBT	SBR	
Lane Configurations	ሻ	7	ሻ	^		^	7	
Traffic Vol, veh/h	57	165	56	877		1132	15	
Future Vol, veh/h	57	165	56	877		1132	15	
Conflicting Peds, #/hr	0	0	0	0		0	0	
Sign Control	Stop	Stop	Free	Free		Free	Free	
RT Channelized	-	None	-	None		-	None	
Storage Length	0	150	250	-		-	75	
Veh in Median Storage, #	0	-	-	0		0	-	
Grade, %	0	-	_	0		0	_	
Peak Hour Factor	84	84	87	87		86	86	
Heavy Vehicles, %	2	2	2	2		2	2	
Mvmt Flow	68	196	64	1008		1316	17	
Major/Minor	Minor2		Major1			Major2		
Conflicting Flow All	1949	658	1316	0		-	0	
Stage 1	1316	-	-	-		_	-	
Stage 2	633	_	_	_		_	_	
Critical Hdwy	6.84	6.94	4.14	_		_	_	
Critical Hdwy Stg 1	5.84	-	-	_		_	_	
Critical Hdwy Stg 2	5.84	_	_	_		_	_	
Follow-up Hdwy	3.52	3.32	2.22	_		_	_	
Pot Cap-1 Maneuver	~ 56	407	521	_		_	_	
Stage 1	215	-	-	_		_	_	
Stage 2	491	_	_	_		_	_	
Platoon blocked, %	171			_		_	_	
Mov Cap-1 Maneuver	~ 49	407	521	_		_	_	
Mov Cap-2 Maneuver	~ 49	-	-	_		_	_	
Stage 1	215	_	_	_		_	_	
Stage 2	431	_	_	_		_	_	
Stage 2	101							
Approach	EB		NB			SB		
HCM Control Delay, s	117.9		0.8			0		
HCM LOS	F							
Minor Lane/Major Mvmt	NBL	NBT EBLn1 EB	Ln2 SBT	SBR				
Capacity (veh/h)	521	- 49	407 -	-				
HCM Lane V/C Ratio	0.124	- 1.385 0.	483 -	-				
HCM Control Delay (s)	12.9		21.8 -	-				
HCM Lane LOS	В	- F	С -	-				
HCM 95th %tile Q(veh)	0.4	- 6.4	2.6 -	-				
Notes								
~: Volume exceeds capaci	tv \$ De	lay exceeds 300	s +· Com	nutation	Not Defined	*: All major v	volume ir	n platoon
. Volamo onoccus cupaci	., Ψ. DC	, 0		r 4.4.1011	. TOL DOMINOU	ai major	. Sidino II	platoon

Intersection							
Int Delay, s/veh	2.6						
Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	4			र्स	M		
Traffic Vol, veh/h	160	1	20	50	2	61	
Future Vol, veh/h	160	1	20	50	2	61	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Free	Free	Free	Free	Stop	Stop	
RT Channelized	-	None	-	None	-	None	
Storage Length	-	-	-	-	0	-	
Veh in Median Storage, #	0	-	-	0	0	-	
Grade, %	0	-	-	0	0	-	
Peak Hour Factor	92	92	92	92	92	92	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	174	1	22	54	2	66	
Major/Minor	Major1		Major2		Minor1		
Conflicting Flow All	0	0	175	0	272	174	
Stage 1	-	-	-	-	174	-	
Stage 2	-	-	-	-	98	-	
Critical Hdwy	-	-	4.12	-	7.12	6.22	
Critical Hdwy Stg 1	-	-	-	-	6.12	-	
Critical Hdwy Stg 2	-	-	-	-	6.12	-	
Follow-up Hdwy	-	-	2.218	-	3.518	3.318	
Pot Cap-1 Maneuver	-	-	1401	-	680	869	
Stage 1	-	-	-	-	828	-	
Stage 2	-	-	-	_	908	-	
Platoon blocked, %	-	-		_			
Mov Cap-1 Maneuver	-	-	1401	_	672	869	
Mov Cap-2 Maneuver	-	-	-	_	672	-	
Stage 1	-	-	-	_	828	-	
Stage 2	-	_	-	_	893	_	
					3.3		
Approach	EB		WB		NB		
HCM Control Delay, s	0		2.2		9.5		
HCM LOS					А		
Minor Lane/Major Mvmt	NBLn1 EBT	EBR	WBL WBT				
Capacity (veh/h)	861 -	-	1401 -				
HCM Lane V/C Ratio	0.08 -		0.016 -				
HCM Control Delay (s)	9.5 -	-	7.6 0				
HCM Lane LOS	Α -	-	A A				
HCM 95th %tile Q(veh)	0.3 -	-	0 -				
2(1011)			-				

Intersection							
nt Delay, s/veh	42.6						
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	W		ሻ	† †	† †	7	
Traffic Vol, veh/h	17	101	198	1232	1370	68	
Future Vol., veh/h	17	101	198	1232	1370	68	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	_	None	
Storage Length	0	-	250	_	_	75	
Veh in Median Storage, #		_		0	0	-	
Grade, %	0	_	_	0	0	_	
Peak Hour Factor	69	69	89	89	98	98	
Heavy Vehicles, %	2	2	2	2	2	2	
Mvmt Flow	25	146	222	1384	1398	69	
VIVIII I IOW	25	140	222	1304	1370	07	
Major/Minor	Minor2		Major1		Major2		
Conflicting Flow All	2535	699	1398	0	-	0	
Stage 1	1398	-	-	-	_	-	
Stage 2	1137	_	_	_	_	_	
Critical Hdwy	6.84	6.94	4.14	_	_	_	
Critical Hdwy Stg 1	5.84	-	-	_	_	_	
Critical Hdwy Stg 2	5.84	_	_	_	_	_	
Follow-up Hdwy	3.52	3.32	2.22	_	_	_	
Pot Cap-1 Maneuver	~ 22	382	485	_	_	_	
Stage 1	194	-	-	_	_	_	
Stage 2	268	_	_	_	_	_	
Platoon blocked, %	200			_	_	_	
Mov Cap-1 Maneuver	~ 12	382	485				
Mov Cap-1 Maneuver	~ 12	302	405	_	_	_	
Stage 1	194	-	-	-	-	-	
	145	-	-	-	-	-	
Stage 2	143	-	-	-	-	-	
Approach	EB		NB		SB		
HCM Control Delay, s	\$ 783.6		2.6		0		
HCM LOS	F						
dinor Long/Major Missel	NIDI	NDT CDI ~1	CDT CDD				
Minor Lane/Major Mvmt	NBL	NBT EBLn1	SBT SBR				
Capacity (veh/h)	485	- 70					
HCM Lane V/C Ratio	0.459	- 2.443					
HCM Control Delay (s)	18.6	-\$ 783.6					
ICM Lane LOS	С	- F					
HCM 95th %tile Q(veh)	2.4	- 16.5					
lotes							
: Volume exceeds capa	city \$: De	lay exceeds 30	00s +: Com	putation N	lot Defined *: All major v	volume in r	olatoon

Intersection								
nt Delay, s/veh 10	0.7							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	7	7	*	^	^	7		
Traffic Vol, veh/h	17	101	198	1232	1370	68		
Future Vol, veh/h	17	101	198	1232	1370	68		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None .	-	None	-	None		
Storage Length	0	150	250	-	-	75		
Veh in Median Storage, #	0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	69	69	89	89	98	98		
Heavy Vehicles, %	2	2	2	2	2	2		
Mvmt Flow	25	146	222	1384	1398	69		
Major/Minor	Minaro		Ma:ar1		Ma!0			
Major/Minor	Minor2	/00	Major1		Major2	^		
Conflicting Flow All	2535	699	1398	0	-	0		
Stage 1	1398	-	-	-	-	-		
Stage 2	1137	-	-	-	-	-		
Critical Hdwy	6.84	6.94	4.14	-	-	-		
Critical Hdwy Stg 1	5.84	-	-	-	-	-		
Critical Hdwy Stg 2	5.84	-	-	-	-	-		
Follow-up Hdwy	3.52	3.32	2.22	-	-	-		
Pot Cap-1 Maneuver	~ 22	382	485	-	-	-		
Stage 1	194	-	-	-	-	-		
Stage 2	268	-	-	-	-	-		
Platoon blocked, %				-	-	-		
Mov Cap-1 Maneuver	~ 12	382	485	-	-	-		
Mov Cap-2 Maneuver	~ 12	-	-	-	-	-		
Stage 1	194	-	-	-	-	-		
Stage 2	145	-	-	-	-	-		
Approach	EB		NB		SB			
HCM Control Delay, s	178.5		2.6		0			
HCM LOS	F							
Minor Lane/Major Mvmt	NBL	NBT EBLn1 El	BLn2 SBT	SBR				
Capacity (veh/h)	485	- 12	382 -	-				
HCM Lane V/C Ratio	0.459	- 2.053 ().383 -	-				
HCM Control Delay (s)	18.6	\$ 1119.3	20.2 -	-				
HCM Lane LOS	С	- F	С -	-				
HCM 95th %tile Q(veh)	2.4	- 3.9	1.8 -	-				
Votes	tu 6. D-	lav avasada 200	00 00	nutotic:	Not Defined *: All mails	volumo in :	alataan	
~: Volume exceeds capacit	ıy \$: De	lay exceeds 300	us +: Com	putation	Not Defined *: All major	volume in	ualuun	

- <u></u>
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WARRANT FOR LEFT-TURN STORAGE LANES ON TWO-LANE HIGHWAY

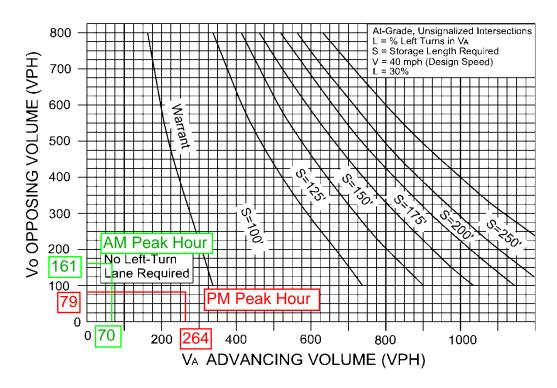


FIGURE 3-9

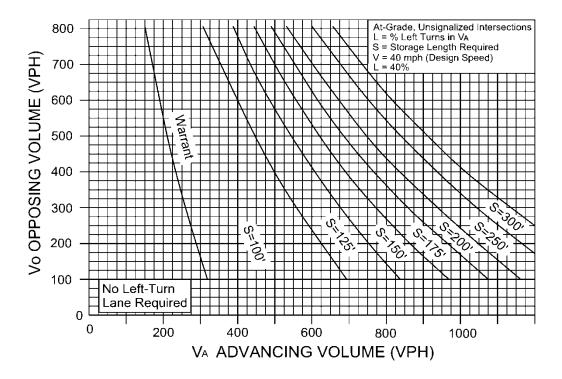
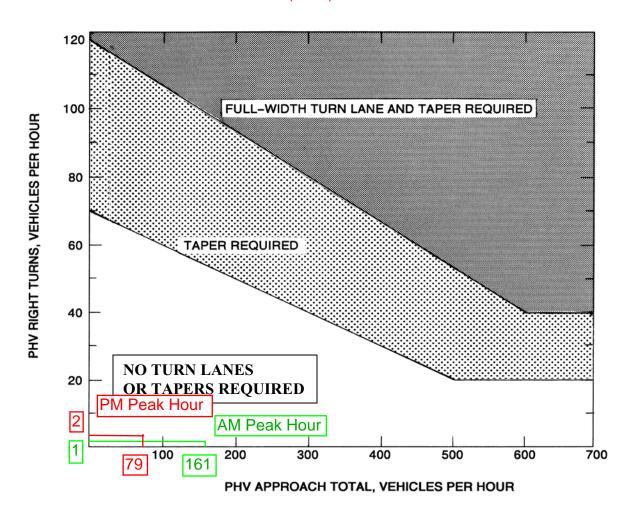


FIGURE 3-10



Appropriate Radius required at all Intersections and Entrances (Commercial or Private).

LEGEND

PHV - Peak Hour Volume (also Design Hourly Volume equivalent)

Adjustment for Right Turns

For posted speeds at or under 45 mph, PHV right turns > 40, and PHV total < 300.

Adjusted right turns = PHV Right Turns - 20

If PHV is not known use formula: PHV = ADT x K x D

K = the percent of AADT occurring in the peak hour

D = the percent of traffic in the peak direction of flow

Note: An average of 11% for K x D will suffice.

When right turn facilities are warranted, see Figure 3-1 for design criteria.*

FIGURE 3-26 WARRANTS FOR RIGHT TURN TREATMENT (2-LANE HIGHWAY)

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^{*} Rev. 1/15



4343 Cox Road Glen Allen, VA 23060

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Exhibit 1 – Looking west on Holly Corner Road approximately 275 feet east of the proposed driveway location



Exhibit 2 – Looking east on Holly Corner Road approximately 275 feet east of the proposed driveway location





PRE-SCOPE OF WORK MEETING FORM

Information on the Project Traffic Impact Analysis Base Assumptions

The applicant is responsible for entering the relevant information and submitting the form to VDOT and the locality no less than three (3) business days prior to the meeting. If a form is not received by this deadline, the scope of work meeting may be postponed.

Contact Information									
Consultant Name:	Ramey Kemp & Ass	Ramey Kemp & Associates, Inc. / Carl Hultgren, P.E., PTOE							
Tele:	(804) 217-8560								
E-mail:	chultgren@rameyker	chultgren@rameykemp.com							
Developer/Owner Name:		ting Services, PLC / S	amer Shalaby, P.E.						
Tele:	(540) 368-1327								
E-mail:	sshalaby@dev-consu	halaby@dev-consulting.com							
Project Information									
Project Name:	Holly Corner		Locality/County:	Stafford County					
Project Location: (Attach regional and site specific location map)	Refer to Figure 1								
Submission Type	Comp Plan 🗌	Rezoning 🖂	Site Plan 🖂	Subd Plat					
Project Description: (Including details on the land use, acreage, phasing, access location, etc. Attach additional sheet if necessary)	mile west of U.S. 17 single-family homes,	ed on the south side of (Warrenton Road). The with one new full-most connection to Country southeast.	ne preliminary plan in vement driveway on l	cludes up to 130 Holly Corner Road,					
Proposed Use(s): (Check all that apply; attach additional pages as necessary)	Residential 🔀	Commercial	Mixed Use	Other					
	Residential Uses(s)								
	Number of Units:	130							
	ITE LU Code(s):	210							
			Other Use(s)						
			ITE LU Code(s):						
	Commercial Use(s)								
	ITE LU Code(s):								
			Independent Variable	(s):					
	Square Ft or Other Va	ariable:							
Total Peak Hour Trip Projection:	Less than 100	100 – 499 🔀	500 – 999	1,000 or more					

Traffic Impact Analy	sis Assumption	S						
Study Period	Existing Year: 201	7	Build-out	t Yea	r: 2022		Design Year: 2022	
Study Area Boundaries	North: See Figure	1		Sout	h:			
(Attach map)	East:			Wes	t:			
External Factors That Could Affect Project (Planned road improvements, other nearby developments)		South -		s wit	h 200 lots l	ouilt ou	it by 2022	
Consistency With Comprehensive Plan (Land use, transportation plan)	allows up to 3.0 lot	isting zoning is A-1, and the proposed rezoning is R-2. The Comprehensive Planows up to 3.0 lots per acre. The proposed site has 128 homes on 48.603 acres, ich is approximately 2.6 lots per acre.						
Available Traffic Data (Historical, forecasts)	,	.S. 17 (Warrenton Road) - 38,000 vpd in 2012 / 39,000 vpd in 2015 olly Corner Road - 1,700 vpd in 2012 / 1,700 vpd in 2015						
Trip Distribution	Road Name: See Figure 2 Road Name:							
(Attach sketch)	Road Name:			Ro	ad Name:			
Annual Vehicle Trip	1.5%				ıdy	\boxtimes /	AM 🛛 PM 🗌 SAT	
Growth Rate:		Peak	Hour of t	he G	enerator			
	1.U.S. 17 (Warrent Holly Corner Road		ad) at	6.				
Study Intersections	2.Holly Corner Ro Driveway	ad at P	roposed	7.				
and/or Road Segments (Attach additional sheets as	3.			8.				
necessary)	4.	West: West:	4.			9.		
	5.			10.				
Trip Adjustment Factors				0				
Software Methodology	Synchro 🗌 H	CS (v.2	2000/+)	aa	aSIDRA [CORS	SIM Other SimTraffic	
Traffic Signal Proposed or Affected (Analysis software to be used, progression speed, cycle length)	Synchro / SimTraf study intersections		vill be used	d to a	nalyze LOS	S, dela	y, and queueing at the	

It is important for the applicant to provide sufficient information to county and VDOT staff so that questions regarding geographic scope, alternate methodology, or other issues can be answered at the scoping meeting.

Improvement(s) Assumed or to be Considered	The need for turn lanes and other off-site improvements will be determined based on the results of the TIA.
Background Traffic Studies Considered	None
Plan Submission	☐ Master Development Plan (MDP) ☐ Generalized Development Plan (GDP) ☐ Preliminary/Sketch Plan ☐ Other Plan type (Final Site, Subd. Plan)
Additional Issues to be Addressed	☑ Queuing analysis ☐ Actuation/Coordination ☐ Weaving analysis ☐ Merge analysis ☑ Bike/Ped Accommodations ☑ Intersection(s) ☐ TDM Measures ☐ Other

NOTES on ASSUMPTIONS:

The TIA will include three analysis scenarios:

- Existing (2017) Traffic Conditions
- No-Build (2022) Traffic Conditions
- Build (2022) Traffic Conditions

SIGNED:

PRINT NAME:

SCOPE OF WORK MEETING CONCLUSIONS

ADDITIONS TO THE VDOT REQUIRED ELEMENTS, CHANGES TO THE METHODOLOGY OR STANDARD ASSUMPTIONS, AND SIGNATURE PAGE

Any additions to the VDOT Required Elements or chan Assumptions due to special circumstances that are appro-	ges to the Methodology or Standard oved by VDOT:
The applicant will contact VDOT and the locality pranalysis study in the event there are any substantial caffect the scope of the study.	ior to the preparation of the traffic impact changes in the existing conditions that will
AGREED: Applicant of Consultant	DATE: 1-10-18
PRINT NAME: Carl Hultgren Applicant og Consultant	
SIGNED: VDOT Representative	DATE: 1/10/18
PRINT NAME: DAVID 2. BEALE VDOT Representative	
SIGNED: Churchs & Dess. Local Government Representative	DATE: 1/11/18
PRINT NAME: Charles J. Hess Local Government Representative	

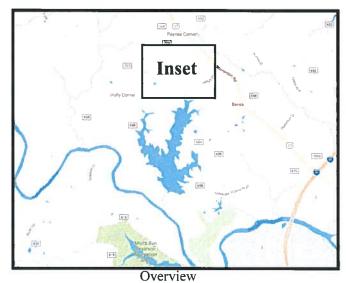
TIA	SCOPING	MEETING	SIGN-IN	SHEET
-----	---------	---------	---------	-------

Project: Holly Corner Meeting Date: 2-8-17

County: Starford Place/Room: VDOT

lot.virginia.gov
@vdot.virginia.gov
y@vdot.virginia.gov
vdot.virginia.gov
nn@vdot.virginia.gov
Dvdot.virginia.gov
well@vdot.virginia.gov
dcountyva.gov
dcountyva.gov
fordcountyva.gov
BESTE9CAOLCOM
a @ ramey komp. com
Frankuna FF-pe.com
Der-consulhing. com
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Inset

LEGEND



Study Intersection



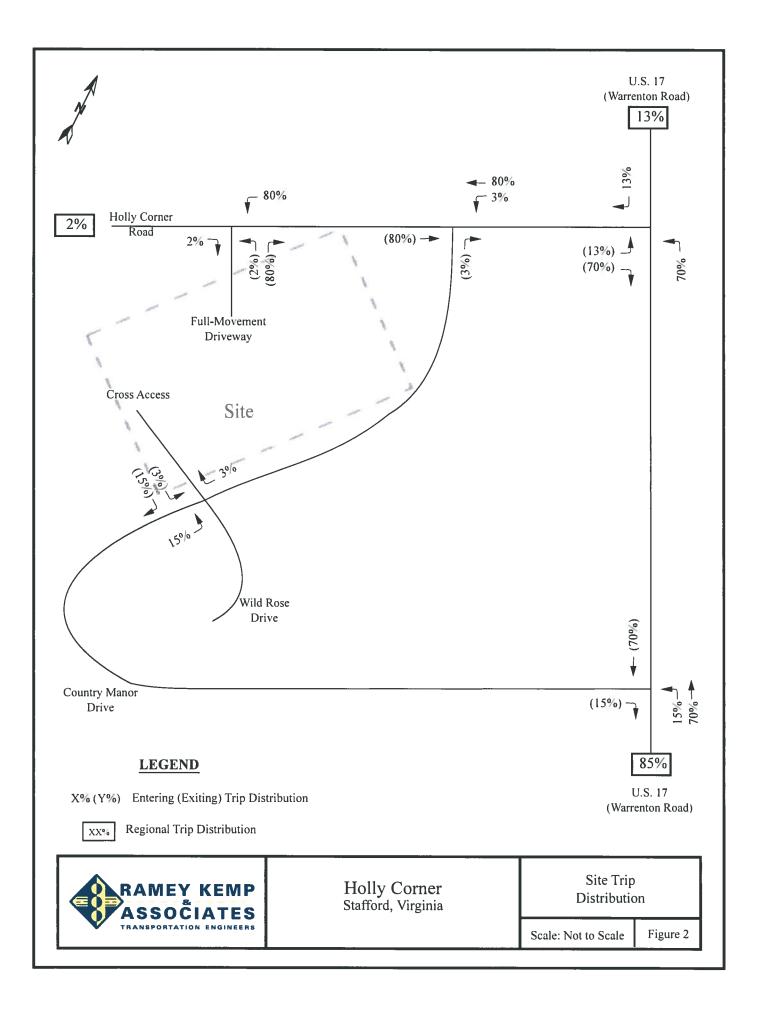
Site Boundary





Holly Corner Stafford, Virginia Site Location and Study Intersections

Scale: Not to Scale





RAMEY KEMP & ASSOCIATES, INC.

4343 Cox Road Glen Allen, VA 23060

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Holly Corner Stafford County, VA

ITE Trip Generation – 9th Edition – Weekday

Land Use (ITE Land Use Code)	Size	•	kday Fraffic od)	AM Peak Hour (vph)		PM Peak Hour (vph)	
		Enter	Exit	Enter	Exit	Enter	Exit
Single Family Detached (210)	130 homes	668	668	25	76	84	49

January 30, 2017



RAMEY KEMP & ASSOCIATES, INC. 4343 Cox Road

Phone: 804-217-8560 Fax: 804-217-8563

Glen Allen, VA 23060 www.rameykemp.com

Holly Corner Approved Developments Stafford County, VA

Table 1 ITE Trip Generation – 9th Edition – Weekday – Westlake Residential

Land Use (ITE Land Use Code)	Size	Weekday Daily Traffic (vpd)		AM Peak Hour (vph)		PM Peak Hour (vph)	
		Enter	Exit	Enter	Exit	Enter	Exit
Single Family Detached (210)	200 homes	952	952	38	112	126	74

Table 2 ITE Trip Generation – 9th Edition – Weekday – Summerset Ridge South

112 111p Colletteton > Lateron // Collety				Summer set Tuage South				
Land Use (ITE Land Use Code)	Size	Weekday Daily Traffic (vpd)		AM Peak Hour (vph)		PM Peak Hour (vph)		
		Enter	Exit	Enter	Exit	Enter	Exit	
Single Family Detached (210)	23 homes	110	110	4	13	14	9	

Table 3 ITE Trip Generation - 9th Edition - Weekday - Holly Refuge

Land Use (ITE Land Use Code)	Size	Weekday Daily Traffic (vpd)		AM Peak Hour (vph)		PM Peak Hour (vph)	
		Enter	Exit	Enter	Exit	Enter	Exit
Single Family Detached (210)	7 homes	34	34	1	4	4	3

February 9, 2017

